

1 **Estimation of Intersection Turning Movement Flows with NETFLOCON: Weight**
2 **Constraint Calibration**

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6
7 **Abstract**

8 Traffic management strategies grounded on reliable traffic demand information can reduce
9 congestion-related costs. While traffic video images can supply real-time turning movement (TM)
10 information, their cost can be prohibitive, especially to smaller agencies. The NETFLO algorithm
11 utilizes Minimum Cost Flow optimization by generating network flows, from a sparse set of
12 detected link flows. NETFLO construct (NETFLOCON) applies an optimal constraint protocol to
13 the NETFLO algorithm inferring reliable TM estimates from link flows detected in quasi real-
14 time. The research goal is to establish consistently robust weight constraints that enable reliable
15 estimation of urban intersection TM. The optimal weighting scheme consistently outperforms
16 weight sets found in the literature, for estimating 5-min TM flows. The results yield a mean
17 coefficient of determination (R^2) of 93% between observed and modeled TM flows. Mean 5-min
18 Root Mean Square Error (RMSE) is 8.9 ± 0.4 vehicles, while the Pearson correlation coefficient (r)
19 is 0.96 ± 0.01 . The repeatable control regime that restrains the NETFLO optimization algorithm is
20 an algorithm construct, NETFLOCON. And this paper shows that NETFLOCON is a reliable
21 urban intersection movement estimator.

22 **Keywords**

23 NETFLO construct, turning movement estimation, weight constraint calibration, intersection
24 management, traffic demand

25 **Introduction**

26 Equilibrium between available infrastructure and user demand is a prerequisite for an economical
27 and efficient transportation system. However, traffic congestion is a growing problem, that
28 diminishes transportation efficiency. Each U.S. driver lost 97 hours in 2018 due to congestion,
29 incurring the cost of \$1,348. (Reed, 2019) Traffic annual delay is expected to amount to \$2,300
30 per U.S. commuter- as much as a Boston driver has already paid in 2018. (Centre for Economics
31 and Business Research (CEBR), 2014)

32 Traffic congestion can be improved through proper management strategies- e.g., adapting traffic
33 signals, speed limits or lane use. Also, information on the available spare capacity helps in
34 rerouting vehicles, e.g., during special events or road closures. But, to identify the efficient
35 management strategy, one needs to have the reliable and timely information on the demand. (Grant,
36 et al., 2011)

37 The most significant transportation delay component in urban areas arises at intersections since
38 the capacity here is the lowest. (Denney Jr., Curtis, & Olson, 2012) Intersections consist of multiple
39 approach and departure legs, which should accommodate travel demand. For example, a vehicle
40 approaching a 4-legged intersection can take one of the three departure legs by making left turn,
41 right turn, or straight-on maneuver. Twelve known turning movement (TM) flows associated with
42 each possible maneuver identify the intersection demand.

43 We have traffic detectors on many of our urban traffic networks, but we lack comprehensive state
44 estimation algorithms. While video image detection can supply good estimates, many agencies
45 lack the resources to deploy video to all intersections. So, even in the most instrumented networks,
46 we lack comprehensive real-time flow detectors. The goal of this paper is to present a methodology
47 for the development of a real-time state estimator, that draws on a sparse set of detector data.

48 The NETFLO algorithm utilizes Minimum Cost Flow (MCF) optimization by generating network
49 flows in one run. (Kennington & Helgason, 1980) NETFLO algorithm was originally written in
50 FORTRAN but was modified for compatibility with Unix Operating System in 1991. (Johnson,
51 1993) The algorithm sends flow through the network at a minimum total cost, with respect to
52 limitations, allowing infeasibility along the optimization process. (Hillier & Lieberman, 2010)
53 However, NETFLO algorithm is merely an instrument, holding no value in estimating TM, unless
54 properly constrained.

55 NETFLO construct (NETFLOCON) applies the optimal constraint protocol to the NETFLO
56 algorithm, inferring reliable estimates from sparse detected link flows. (Martin, 1995)
57 NETFLOCON assesses the state of the network travel demand and returns unknown flows.
58 Network flows satisfy the series of linear equations allowing flow continuity, resulting in TM and
59 link flow estimates. All network flows comply with the constraints of detected flows and spatial
60 geometry.

61 Mid-block detectors continuously collect sparse network flows. Detector coverage of less than
62 15% is sufficient for reliable NETFLOCON estimation of TM flows. (Martin, 1995) Recurrent
63 detected flows update the algorithm input, producing the quasi real-time (e.g., 5-minute) flow
64 estimates. Ideally, algorithm TM estimates should match the real TM flows acquired from stop-

65 line detectors. Available intersection demand could help traffic engineers to monitor intersection
66 performance, implement strategic control and maximize efficiency.

67 The research establishes consistent and robust NETFLOCON constraints that enable reliable
68 estimation of urban intersection TM flows in quasi real-time. NETFLOCON can be constrained in
69 many ways: through upper and lower bounds, weights and error arc factor. Different constraint
70 structures produce different flow estimates, and consequently- different model reliability.
71 Algorithm objective function integrates weight protocol, so the NETFLOCON estimates'
72 reliability depends on properly chosen weight constraints. (Martin, 1995)

73 Weight constraints represent costs of traveling through network links. "Expensive" maneuvers
74 discourage TM flows, as cost and demand are associated. Intersection approach volume splits
75 between turning directions according to the weight protocol. Therefore, optimal weight procedure
76 (W^*) should depict intersection turning proportions to bind TM estimates to the real flows.

77 The literature is silent on efforts to thoroughly assess weight constraint formulation and advance
78 performance of NETFLOCON in estimating intersection TM flows. The Theoretical Network is
79 used to systematically calibrate NETFLOCON weight constraint protocol. Established turning
80 proportions produce sets of real and estimated TM flows, generating certain model reliability.
81 Maximum average coefficient of determination (R^2) for all turning ratios points at W^* . W^* is then
82 validated on a real network during the peak hour.

83 The methodological objectives that will address the overarching research goal are as follows:

- 84 1. Determine how the ratio between left (L) and right (R) TM weights influences R^2 of TM
85 flows and identify the optimal L:R weight proportion.

- 86 2. Determine how multiplying weights by a factor influences R^2 and identify the optimal
87 multiplication (m) factor.
- 88 3. Determine how manipulating through (T) TM weight influences R^2 and identify the
89 optimal T weight.
- 90 4. Determine how manipulating error arc (E) weight influences R^2 and identify the optimal E
91 weight.
- 92 5. Identify the optimal weight constraint protocol (W^*) by combining L:R weight proportion,
93 m factor, T and E weights. Assess performance of W^* on a real network and compare it to
94 the weight sets suggested in the literature.

95 Decades ago, the time required for algorithm solution was determining factor, dependent on
96 network complexity or the processor power. (Ahuja, Magnanti, & Orlin, 1993) A single execution
97 of NETFLO algorithm lasted for hours. Nowadays, an average computer configuration can
98 produce the estimates instantly, so NETFLOCON can be estimated thoroughly and efficiently. In
99 addition, on-line estimation can now be accomplished practically, rather than just theoretically.
100 So, the lack of NETFLOCON advancement for over 20 years can and needs to change.

101 The paper is organized as follows: prior studies are synthesized to show that NETFLO algorithm
102 is reliable, repeatable and replicable. The average intersection turning proportions are set to
103 comply with the literature recommendations. Constraint tightening, and validation approach are
104 explained in detail. Findings and conclusions are then reported. Finally, research limitations and
105 proposed and future directions specified.

106 **Literature Review**

107 Direct collection of TM flows is complex, expensive and slow. (Bert, 2009) Static and dynamic
108 models are regularly used for estimation. Static models total the demand over a long period (e.g.,
109 peak hour), while dynamic models are continuously fed data and update TM flows in real-time.
110 (Bera & Rao, 2011) Most real-time estimation algorithms, although reliable, assume that
111 intersection inflows and outflows are available. (Chang & Tao, 1998) (Ghanim & Shaaban, 2019)
112 Alternatively, models suffer from insufficient constraints, providing unreliable estimates. (Cremer
113 & Keller, 1987) (Nihan & Davis, 1987) (Bell M. , 1991) (Chang & Wu, 1994) (van der Zijpp,
114 1996)

115 Processed images from camera-based vision systems became popular in acquiring traffic data.
116 (Knoop, Hoogendoorn, & van Zuylen, 2009) (Yang & Pun-Cheng, 2018) For example, Miovision
117 software requires collecting recording from scout pole units, uploading it to the website, then
118 accessing the platform to access actual traffic statistics. (Shiravi, 2019) However, this is not
119 suitable for real-time data extraction. Data rich hardware platforms can be engineered to extract
120 real-time high-resolution data and generate Automated Traffic Signal Performance Measures
121 (ATSPM). (Huang, et al., 2018) But, these methods require specific equipment, permits, and
122 technical installation.

123 The future of TM data could rely on some of the beforementioned algorithms and technologies,
124 paired with information from connected vehicles with Vehicle-to-Infrastructure communication.
125 This way, TM information would be extensive and highly reliable. Yet, most of the agencies
126 currently do not possess resources to equip an entire network. Volume data remain relatively sparse

127 on the majority of the arterial networks, although the need for TM information persists . (Shoup,
128 Remias, Hainen, Grimmer, & Davis, 2013)

129 The NETFLO algorithm follows the Kirchhoff's Law and flow limitations on the arcs, just as MCF
130 algorithm. The network flow moves from source (supply) nodes to sink (demand) nodes in the
131 most cost-effective manner and produces specific arc flows as an output. The model solves the set
132 of constraints and minimizes the objective function. The network is directed and connected, and
133 vehicles travel through its i nodes and j arcs. Traffic flow cannot be negative, or above the road
134 capacity.

135 The objective function takes the form of:

136
$$\min \rightarrow \sum w_j x_j$$

subject to:

137
$$0 \leq l_j \leq x_j \leq u_j, \text{ for each arc } j$$

$$\sum x_j (\text{outflow}) - \sum x_j (\text{inflow}) = b_i, \text{ for each node } i$$

where:

w_j - weight (cost) per unit flow through arc j

x_j - flow vector through arc j (decision variable)

138 l_j - arc lower bound

u_j - arc upper bound

b_i - net flow generated at node i

139 The necessary condition for a feasible MCF problem is to ensure network is balanced- the sum of
140 flows generated at all nodes equals zero ($\sum b_i = 0$). However, having the specific arc flow
141 information from detectors ($l_j = x_j = u_j$), inevitably creates an infeasible solution. Introducing
142 flexibility through error arcs can successfully overcome this. (Bell M. C., 1987) Each detector arc

143 has two parallel and opposite error arcs, to allow movement of vehicles in both directions. Figure
144 1 shows arc types: detector, error, link, and TM arcs.

145 Error arcs need to be constrained through its lower and upper bound, to inhibit the artificial flows
146 and improve estimation reliability. The capacity of error arcs is defined as a product of error arc
147 factor (φ) and the corresponding detected flow. The experiment on the Leicester, England network
148 has shown that the value of φ should be 0.025. (Martin & Bell, 1992) Upper bound of non-error
149 arcs is defined through Highway Capacity Manual (HCM) capacity guidelines and their lower
150 bound is set to 0.

151 NETFLOCON can be defined as quasi-dynamic. The period flows are aggregated, but time slices
152 record demand fluctuations. However, analyzed periods should be independent, since queue
153 discharge cannot be modeled. Therefore, periods have to follow two requirements: be as short as
154 possible to approximate real-time flows and be long enough to preserve interval independence.
155 Five-minute periods are viable to satisfy both. (Martin, 1995) (Martin, 1997)

156 NETFLOCON's performance was tested on the Leicester, England network for 5-min flows during
157 the afternoon peak. (Martin, 1997) This corresponds to the real-time application. Overall mean R^2
158 was 79-82% between observed and modeled TM flows. However, the flows with lower magnitude
159 were not reliably inferred. The model is shown to be repeatable and relatively reliable.

160 NETFLOCON is shown to be replicable when tested on San Luis Obispo (SLO), California,
161 U.S.A. network. (Martin, 1995) Detector coverage of 15% confirmed that minimum number of
162 detectors is sufficient for reliable model performance. The model R^2 reached 90% for 5-min
163 intervals. Also, this research shows that weighting constraint is desirable, to differentiate the
164 difficulties of the various maneuvers.

165 The SLO network experiment enforced weight constraints, which improved estimation reliability.
166 (Martin, 1995) Mean overall R^2 was 84.1-90.6% for non-weighted, and 92.3-94.7% for weighted
167 regimes. However, only four different weight schemes were tested, so systematic assessment of
168 weight protocols is needed. Additionally, relating intersection turning proportions with weight
169 constraints might improve NETFLOCON performance.

170 Intersection turning ratios are considered as critical input in estimating TM flow estimates and are
171 generally acquired through field or other historical data. (van Zuylen, 1979) The better the
172 information on turning proportions, the more accurate the TM estimation becomes. When
173 unavailable, researchers are inclined to rely on average turning proportions to produce algorithm
174 estimates. (Schaffer, 1988)

175 Left turn is a cross-flow in most countries, so is generally more demanding and less safe than other
176 intersection maneuvers. Intersection turning proportions can be attributed to different factors, but
177 the most influential are intersection inflow and outflow road type, and location in the urban area.
178 Although these factors might increase the portion of left turns, average turning proportions for the
179 entire network remain relatively predictable. Metropolitan Toronto is used to produce proportion
180 recommendations according to the approach and exit road type (arterial-collector combinations).
181 (Hauer, Pagitsas, & Shin, 1981) Average right turn proportions were larger than or equal to the
182 left turning proportions for all of the cases considered. HCM recommends using 10% for left and
183 10% for right turn ratios. (Transportation Research Board, 2010)

184 NETFLOCON has performed well on two networks, showing that the algorithm can reliably
185 estimate intersection demand. Weight constraint protocol improves estimation reliability.

186 However, weight constraints have not been systematically tested to identify the optimal constraint
187 regime. (Martin, 1997)

188 Other methods of acquiring TM are image processing, and other detection algorithms. Video
189 detection is expensive. Other detection estimation algorithms need comprehensive link detection
190 to deliver reliable estimates. NETFLOCON can estimate TM flows with no additional investment
191 and from sparse network flows.

192 **Methodology**

193 Calibration of NETFLOCON is used to adjust the weight constraints, so model outputs can better
194 portray “real” TM flows for different intersection turning proportions. The Theoretical Network is
195 established, as it allows choosing turning ratios arbitrarily. Each tested intersection proportion
196 produces a set of “real” and estimated TM flows. Paired flow sets yield coefficient of determination
197 (R^2), measuring the strength of the linear relation between estimated and real TM flows. Mean R^2
198 for all turning ratios is maximized, to identify the optimal weights W^* . Calibrated NETFLOCON
199 is then applied to the real network to confirm its predictive power and demonstrate validity.

200 *Calibration*

201 Actual TM flows are subject to intersection approach flow and intersection turning ratio. Similarly,
202 estimated TM flows depend on the estimated approach flow and weight constraint protocol.
203 Approach flow is predictable from network topology, parking data and detected flows. Observed
204 and estimated TM yield R^2 , which is used to select W^* .

205 Intersection turning proportions are the percentage of intersection approach flow engaging in
206 certain intersection maneuver. For example, approach volume of 1,000 vehicles and turning ratio

207 10:70:20 would direct 100 vehicles left, 700 through and 200 right. However, turning proportions
208 often differ over time and for different intersection approaches. As historical data are not a valid
209 algorithm input, W^* needs to be robust and applicable to different unknown turning proportion
210 settings. For instance, model should produce reliable estimates for both 5:90:5 and 10:60:30
211 turning ratios.

212 All variables are held fixed, while only weights are varied. We monitor changes in decision
213 variable R^2 as it changes unambiguously due to weights changed. Procedure is repeated for
214 different intersection turning proportions, producing a set of R^2 weight-dependent values.
215 Analyzed turning proportions assume either right turn dominant flows, or equally represented right
216 and left flows. Maximum mean R^2 for all turning ratios identifies W^* .

217 R^2 represents the fraction of variation explained by the model and takes values between 0 and
218 100%. An R^2 value close to 100% means that the model has a strong correlation between observed
219 and modeled TM flows. Perfect fit ($R^2=100\%$) would incline all points to fall on a line of perfect
220 correspondence (1:1), passing through the origin. High R^2 advocates small and unbiased difference
221 between estimates and observations.

222 We manipulate turning ratios and weight sets on the Theoretical Network based on a real site in
223 Las Cruces, New Mexico, U.S.A. It consists of 15 (7 four-legged and 8 three-legged) intersections.
224 The Theoretical network has 148 nodes and 274 (220 regular and 54 error) arcs. Therefore, two
225 error arcs are added to each of the 27 detector arcs. Detector arcs are fixed, so lower and upper
226 bounds match the detected flows ($l_j=x_j=u_j$). Non-detector arcs have lower bound set to 0. Error arcs
227 have an upper bound of $\phi*x_j$, where error arc factor ϕ is optimized. The remaining arcs have
228 capacity of 250 vehicles/period. Topology of the Theoretical Network is presented in Figure 2.

229 The “real” intersection inflows are uniformly distributed $U(5,200)$ in Microsoft Excel and fixed.
230 Intersection outflows match the inflows, so the Kirchhoff’s law is preserved. Different turning
231 proportions vary sets of TM flows. Only 27 (about 12%) of links are modeled as “detected” flows
232 for NETFLO input. The non-TM flows and external loading are calculated through Kirchhoff’s
233 law. Balancing external loading (BEL) is used to complement flows generated at all nodes, so that
234 feasibility requirement $\sum b_i=0$ holds. Methodology flowchart is shown in Figure 3.

235 The weight constraints follow four hierarchical steps:

- 236 1) Left to right weight proportion,
- 237 2) Multiplication factor,
- 238 3) Through weight, and
- 239 4) Error arc weight.

240 The weight assessment initially sets weights of all arcs to 1. The weights of link and detector arcs
241 remain 1 throughout the process. The error arc weights vary. The hierarchy assumes that step 1
242 addresses the primary influence, namely left to right split. For each test, within each step, we
243 measure the correlation between observed and modeled flows. The step finishes by selecting the
244 best correlation from all tests. This gives the step weight that is applied throughout the subsequent
245 step. So, with the best weight from the previous step, the current step tests a fresh factor.

246 Step 1 seeks the optimal proportion of weights between left and right TM, by analyzing 19 different
247 ratios. In step 2, the multiplication factor m brings consistency to the split. In step 3, the
248 manipulation of through movements relies on the improvement of the correlation between the
249 observed and modeled flows. In step 4, error arc weighting deters NETFLOCON from assigning

250 flows. In this way, NETFLOCON strives to diminish error arc flow which represents the
251 corruption of measured detector flows.

252 *Validation*

253 The optimal weight constraints are validated on a real network in Orem, Utah, U.S.A. Orem is the
254 fifth-largest city in Utah, located about 45 miles South of Salt Lake City, populated by about
255 98,000 people in 2017. (U.S. Census Bureau, 2018) This network is chosen as it encompasses
256 principal arterials, minor arterials and urban collectors. The analyzed area is bounded by 800W
257 Street in the West and 800E Street in the East, 400N in the North, and 800S Street to the South.
258 The study area is shown in Figure 4.

259 Automated Traffic Signal Performance Measures (ATSPM) website contains high-resolution data
260 from each signalized intersection, collected from controllers and traffic probes. (Utah Department
261 of Transportation (UDOT), 2019) (Federal Highway Administration (FHWA), 2019) Information
262 from the website is used as the algorithm input: sparse traffic flows, signal settings, external
263 network loading and TM counts to correlate real and estimated flows. Number of lanes and turning
264 options are detected in Google Maps.

265 The Orem network consists of 12 signalized intersections, with 128 nodes and 208 arcs, as shown
266 in Figure 5. Flow detectors are placed on 12.3% of the network, which is 26 arcs. Therefore,
267 NETFLOCON implements additional 52 error arcs, 260 in total. We analyze 5-min traffic demand
268 on Wednesday, May 3rd, 2017 during the afternoon peak hour (4:50-5:50). Each of the 12 modeling
269 periods produce different R^2 for analyzed weight sets.

270 The 5-minute sparse traffic flows and network supply and demand information are used to produce
271 complete network flow estimates through NETFLOCON. Error arc factor is established as one
272 fortieth of the corresponding detector arc ($\phi=0.025$). Lower bound is set to 0 ($v=0$) for all arcs,
273 and the upper bounds (u) are identified through HCM 2010 equations.

274 Weight constraints are the only ones varied. Five different weight sets are assessed: four proposed
275 in the literature on the SLO network (Martin, 1995), and the fifth- optimal weight set (W^*)
276 extracted from the Theoretical Network calibration. The model is validated through three
277 parameters: Coefficient of determination (R^2), Pearson correlation coefficient (r) and Root Mean
278 Square Error (RMSE).

279 R^2 is a commonly used quantitative model assessment parameter and United States Department of
280 Transportation recommends values of at least 88% for travel model validations. (Cambridge
281 Systematics, Inc., 2010) Overall flows represent 144 pairs of observed-modeled TM flows,
282 containing 48 pairs of each TM direction (left, through and right). R^2 is reported for overall TM
283 flows and each TM separately, for twelve 5-min periods. Treating validation tests as rigorously as
284 calibration tests is adequate, so reporting only R^2 for both calibration and validation is sufficient.
285 (Hollander & Liu, 2008) Coefficient of determination is used as a decision parameter in the
286 calibration, but additional estimation quality indicators can further support conclusions, so are used
287 in the validation.

288 Pearson correlation coefficient (r) is reported as it supplements R^2 in measuring linear correlation
289 between variables. Extreme values ± 1 of r correspond to data points lying exactly on a line, while
290 value of 0 represents no linear correlation. Pearson correlation is a popular measure of goodness-
291 of-fit, but it provides no additional information on the nature of the error. (Hourdakakis,

292 Michalopoulos, & Kottommannil, 2003) It can equally penalize small and large errors, even
293 though minor variations around the mean are in the nature of traffic phenomena.

294 On the other hand, RMSE depends on the squared difference between observed and estimated TM
295 flows, and hence places a higher penalty on large errors. RMSE denotes a deviation of unexplained
296 variance. It is convenient as it quantifies estimation error within the same scale (units) as a variable.
297 (Chen, Chootinan, Ryu, Lee, & Recker, 2012) (Lu, Rao, Wu, Guo, & Xia, 2015) However,
298 following specific RMSE target values would not be applicable here, as we are using short intervals
299 and very low and unstable traffic flows.

300 The process time of employing the algorithm is expressed through actual Central Processing Unit
301 (CPU) time in milliseconds (ms). This value consists of two components: time used in executing
302 the specific process and amount of CPU time spent in the kernel within the process. The actual
303 CPU time is extracted directly from the Linux command line window, while running NETFLO on
304 a desktop computer.

305 **Results**

306 *Calibration*

307 Arc weights are calibrated through four steps: left to right turning movement proportion,
308 multiplication factor (m), through turning movement and error arc factor. The optimal weights
309 from each step are transferred to the next. The output from step 4 represents the optimal weight
310 set W^* . The mean R^2 values for all turning proportions are presented in Figure 6.

311 Initial R^2 for non-weighted regime is 52.9%. The first calibration step suggests that weight
312 proportion between left and right TM weights should be 5:4, resulting in 85.8% reliability. The

313 mean R^2 slightly grows as multiplication factor increases in step 2, reaching stability for $m=12$.
314 So, step 2 advocates that left TM weight should be 60, while right TM weight should be 48. Step
315 3 evaluates if through TM arc weight should be changed. However, through weight should remain
316 1, as there is no estimation quality improvement when it increases. Similar conclusion is reached
317 in step 4 for error arc weight. Therefore, optimal weights are 60 for left TM, 48 for right TM, and
318 1 for all the other arcs.

319 *Validation*

320 Validation utilizes the real network in Orem, Utah, U.S.A. applying four initial weights (W_0 , W_1 ,
321 W_2 and W_3) used on SLO network and optimal weight W^* . (Martin, 1995) The W_0 is the base,
322 non-weighted hierarchy. The logic applied to the weighted regimes was that the order of driving
323 difficulty is: driving through a link was the simplest, intersection through movement, left-
324 protected, right-unprotected, followed by left-permitted maneuver, which is the most complex.
325 Error arcs were the most weighted, being approximately 3 times the value of the next most
326 expensive arc.

327 The optimal weights (W^*) extracted from the Theoretical Network calibration also inferred R^2 .
328 The optimal weight set uses 60 for left TM and does not differ if turn is permitted or protected.
329 Right TM weight is 48, while error, link and through weights are 1. Table 1 summarizes coefficient
330 of determination for twelve 5-min periods and 5 weight sets presented in Table 2.

331 Estimation reliability when using optimal weight set (W^*) outperforms use of all the other
332 analyzed weight protocols (W_0 , W_1 , W_2 , W_3). Mean R^2 value for optimal weights is $92.7 \pm 1.0\%$
333 ranging from 90.4-94.1%. The second-best estimation is for W_2 weight set, inducing $90.2 \pm 1.4\%$
334 reliability and ranging from 87.4-92.8%. Weight protocols W_1 and W_3 achieved mean R^2 of

335 87.1±2.4% and 88.4±2.0%, respectively. Non-weighted regime W0 was the least reliable with R²
336 of 78.8±2.8%.

337 The estimation reliability for twelve 5-min periods is shown in Table 3. Three parameters are
338 calculated for optimal weights (W*) and combined TM: coefficient of determination (R²), Root
339 Mean Square Error (RMSE) and Pearson correlation coefficient (r). Also, R² is presented for each
340 of the TM.

341 Mean 5-min R² for all TM during P.M. peak hour is 92.7±1.0%, reaching 94.1%. Almost all
342 through movement (dominant) flows are correctly predicted (R²=96.7±1.0%). However, left and
343 right intersection turn estimations were not as reliable. Mean R² for right TM is 41.7±13.6% and
344 for left TM 28.5±15.5%. Overall RMSE is 8.9±0.4 vehicles, while r is 0.96±0.01. Mean actual
345 CPU time needed for the NETFLOCON execution is 6±1ms, ranging between 4ms and 9ms.

346 **Discussion**

347 NETFLOCON can quickly produce reliable intersection TM estimates. Differently constrained
348 algorithm yields different TM flow estimations. Consequently, different reliability is incurred
349 when comparing modeled and actual (unknown) TM flows. Identifying the optimal constraint
350 structure is crucial to produce consistent TM flow approximations.

351 The Theoretical Network is used to calibrate weight constraints. The optimal weight protocol must
352 allow reasonable reliability under diverse traffic conditions, since NETFLOCON does not use
353 information from previous periods. Different intersection turning proportions are examined, since
354 they can fluctuate from one period to another. Ten intersection turning proportions are analyzed,
355 and through movement was always prevailing. Right/left turns were either equally represented, or
356 right turn dominated over left.

357 The four-step calibration produced optimal weight (W^*) regime: 60 for left TM, 48 for right TM,
358 1 for all other weights. Optimal weights return the highest mean R^2 for all intersection turning
359 proportions of 86.7%. W^* applied to commonly encountered turning flow distributions can
360 reliably predict TM flows. Unlike found in the literature, error arc weight should be 1, advocating
361 that error arcs do not need to be constrained through weight for reliable results.

362 The Orem, Utah Network validates optimal weight regime attained from calibration. The simulated
363 and actual flows are compared for twelve 5-min intervals during the P.M. peak hour. Straight-on
364 flows are reliably predicted. Almost 50% of right and 30% of left maneuvers are successfully
365 estimated. Figure 7 shows how NETFLO estimates and real flows compare for different TM during
366 one observation period (5:00 P.M.-5:05 P.M.).

367 Left and Right TM flows are not reliably inferred, due to their low values. Even small differences
368 in estimated and observed values produce high errors. Neither Left nor Right turns exceed 40
369 vehicles/5 min, keeping most flows at half of that. So, NETFLOCON cannot be used for precise
370 operations, such as determining the optimal signal timing plan.

371 However, prominent, in this case Through, TM flows can be estimated with high certainty,
372 identifying potentially congested routes. Available spare capacity on the network could guide
373 vehicle rerouting operational decision. NETFLOCON can be used to bridge the gap until
374 automation of data collection becomes feasible everywhere.

375 Left TM flows are low for all arcs and are underestimated by NETFLO. On the other hand, through
376 TM are dominant and are often overestimated. Right TM flows, although low, do not show obvious
377 or consistent irregularity when compared to real flows. While dominant flows can be predicted

378 with high accuracy, estimation discrepancy persists through overestimation and underestimation.
379 Therefore, model might be improved to better capture underlying structure of the data.
380 Value of actual CPU time is very short (less than a second) and encompasses only the startup run-
381 time overhead, caused by simply invoking the algorithm. Therefore, NETFLOCON can produce
382 the output instantaneously, needing negligible computation time. However, computation time
383 should be verified for larger than Orem, Utah Network. This way, the upper asymptotic
384 mathematical notation (the Big O) could be extracted to describe the algorithm's running time as
385 a function of its input complexity.

386 **Limitations**

387 Although reliable, the NETFLO algorithm is limited in three ways. First, it is pseudo-dynamic, so
388 independence between intervals needs to be preserved. Vehicles that remain in the network from
389 the previous period are not accounted for, and queue discharge is not considered. Therefore,
390 modeling intervals cannot be too short. Second, internal source/supply nodes account for parking
391 activity in the network, often estimated from parking capacity and limitations. Finally, the
392 NETFLO algorithm is integer based. Error arc upper bounds need to be rounded up. For example,
393 $\varphi=0.005$ and $\varphi=0.025$ might produce the same capacity constraint for low flows detected. Error
394 arc factor cannot be defined precisely.

395 The main limitation of the Theoretical Network calibration is the use of random uniformly
396 distributed flow values for different TM. "Actual" flows are assigned arbitrary in Microsoft Excel,
397 without considering network topology. Therefore, external node loading for the Theoretical
398 network was much greater than in reality, as flows lacked consistency from node to node.

399 Consequently, error arc factor had to be much larger ($\varphi=0.2$) than if used under real conditions.
400 Capacity of error arcs was relatively high, and the model reliability suffered.
401 Additionally, the Theoretical Network has the same upper bound for all non-error arcs. The
402 Theoretical Network was used only as a setting to infer optimal weight structure, not to suggest
403 use of particular error arc factor or expected TM flow reliability. R^2 values on the Theoretical
404 Network pose no empirical importance relevant to validation. It is merely the best summary of
405 optimal weight set, that needs to be tested on an actual network. Moreover, “real” flows are
406 assigned to the Theoretical Network, so each intersection had the average turning proportion.
407 However, this is not realistic- even consecutive intersections might have completely different turn
408 proportions. This is another reason why real network validation is necessary.

409 **Conclusion**

410 The NETFLOCON estimates intersection TM flows by minimizing the weighted objective
411 function. Both objective function and estimation quality hinge on weight constraints. The
412 Theoretical Network is used to systematically identify the optimal weight constraint formulation.
413 The chosen weights produce the highest mean R^2 for ten average intersection turning proportions.
414 This way, actual turning proportions become irrelevant to NETFLOCON performance, as optimal
415 reliability is reached. The Orem, Utah Network validates the findings by estimating twelve 5-min
416 flows in the afternoon peak hour. The main research findings are as follows:

- 417 1. As higher cost discourages movement through the arc, the cost of left turns should be higher
418 than of right turns. Specifically, optimal proportion between left and right TM ($w(L):w(R)$)
419 should be 5:4.

- 420 2. Estimation quality is maximized after the multiplication factor (m) reaches the value of 5.
421 However, this value should be at least 12 to stabilize R^2 . This means that higher
422 multiplication factors would produce the same results for all turning proportions. The
423 minimum value $m=12$ is chosen as optimal.
- 424 3. Manipulating the weight of through turning movement arcs shows no estimation quality
425 improvement, so optimal through TM weight ($w(T)$) should be 1. This finding supports the
426 premise that the through movement maneuver is the easiest and therefore, the cheapest.
- 427 4. Manipulating the weight of error arcs does not improve estimation estimates. The optimal
428 error arc weight ($w(E)$) should be 1, since weighted error arcs do not enhance algorithm
429 reliability. This suggests that the error arc factor and upper bound limit can sufficiently
430 constrain error arcs and deter error arc flows.
- 431 5. The optimal weight set consistently outperforms previous weighted regimes in the
432 literature for each of the 5-min periods. Optimal weights yield mean R^2 of $92.7 \pm 1.0\%$,
433 while the best performing weight set in the literature reaches $90.2 \pm 1.4\%$. The optimal
434 weights' performance is confirmed as superior, producing higher R^2 , thereby deviating
435 less.

436 Therefore, NETFLOCON can reliably estimate urban intersection TM flows in quasi real-time
437 when constrained by the weight protocol reported. Weight constraint formulation is robust and
438 consistent, when applied to the Theoretical network under different turning proportions.
439 Optimality is first achieved when left turns cost 60 and right turns cost 48 units. But, dominant
440 (through) turning flows are overestimated, while left turns are underestimated. Overestimation of
441 congestion can negatively impact decision making (e.g., needless road construction).

442 Orem, Utah Network has a turning proportion of 15%-70%-15% with up to 4% deviation for each
443 direction. Different turning ratios should be examined to check if overestimation of dominant
444 flows preserves. Also, other networks should be tested to confirm weight protocol repeatability
445 and replicability. Large networks are of particular interest when identifying the computing
446 complexity of the algorithm.

447 Moreover, research should identify how to further constrain NETFLO. This paper confirms that
448 the reliability of TM flows can be improved through deeper weight constraint examination.
449 Nevertheless, there is room for further constraint study. For example, lower and upper bound
450 calibration may diminish left turn underestimation and in turn, decrease the overestimation of
451 intersection through movements.

452 The initial network setup requires identifying network topology and entering predefined
453 constraints. NETFLOCON could then be fed the available detected flows, yielding TM estimates
454 instantly. The new period would supply the new detected values and update the TM flows. Traffic
455 management strategies based on reliable and timely information tend to be successful.
456 NETFLOCON can provide intersection demand information, crucial for alleviating urban
457 congestion. This way, reliable TM estimates can be produced as soon as bottlenecks occur.
458 Consequently, traffic would maximize its efficiency, cost-effectiveness, safety and driver comfort.

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- 464 Ahuja, R. K., Magnanti, T. L., & Orlin, J. B. (1993). *Network Flows: Theory, Algorithm and*
465 *Applications*. Englewood Cliffs, NJ, USA: Prentice Hall.
- 466 Ashok, K., & Ben-Akiva, M. (1993). Dynamic Origin-Destination Matrix Estimation and
467 Prediction for Real-time Traffic Management Systems. *Proceedings, the 12th International*
468 *Symposium of Transportation and Traffic Theory*, (pp. 465-484).
- 469 Bell, M. (1991). The Real Time Estimation of Origin-Destination Flows in the Presence of Platoon
470 Dispersion. *Transportation Research Part B: Methodological*, 25(2-3), 115-125.
- 471 Bell, M. C. (1987). *The Use of Automatic Control Algorithms to Define Urban Traffic Routes*.
472 Durham University.
- 473 Bera, S., & Rao, K. (2011, December). Estimation of Origin-Destination Matrix from Traffic
474 Counts: The State of the Art. *European Transport- Transporti Europei*(49), 3-23.
- 475 Bert, E. (2009). *Dynamic Urban Origin-Destination Matrix Estimation Methodology*. Suisse:
476 ÉCOLE POLYTECHNIQUE FÉDÉRALE DE LAUSANNE. doi:10.5075/epfl-thesis-4417
- 477 Cambridge Systematics, Inc. (2010). *Travel Model Validation and Reasonability Checking*
478 *Manual Second Edition*. Federal Highway Administration.
- 479 Centre for Economics and Business Research (CEBR). (2014). *The Future Economic and*
480 *Environmental Costs of Gridlock in 2030: An Assessment of the Direct and Indirect Economic*
481 *and Environmental Costs of Idling in Road Traffic Congestion to Households in the UK, France,*
482 *Germany and the USA*. Center for Economic and Business Research. Kirkland, WA, U.S.A.:
483 INRIX, Inc. Retrieved from [https://www.ibtta.org/sites/default/files/documents/MAF/Costs-of-](https://www.ibtta.org/sites/default/files/documents/MAF/Costs-of-Congestion-INRIX-Cebr-Report%20(3).pdf)
484 [Congestion-INRIX-Cebr-Report%20\(3\).pdf](https://www.ibtta.org/sites/default/files/documents/MAF/Costs-of-Congestion-INRIX-Cebr-Report%20(3).pdf)
- 485 Chang, G., & Wu, J. (1994). Recursive Estimation of Time-varying O-D Flows from Traffic
486 Counts in Freeway Corridors. *Transportation Research Part B: Methodological*, 28(2), 141-160.
- 487 Chang, G.-L., & Tao, X. (1998). Estimation of Dynamic O-D Distributions for Urban Networks.
488 In B. M. Ayyub, & B. M. Ayyub (Ed.), *Uncertainty Modeling and Analysis in Civil Engineering*
489 (pp. 211-230). Boca Raton, FL: CRC Press LLC.
- 490 Chen, A., Chootinan, P., Ryu, S., Lee, M., & Recker, W. (2012). An Intersection Turning
491 Movement Estimation Procedure Based on Path Flow Estimator. *Journal of Advanced*
492 *Transportation*(46), 161-176. doi:10.1002/atr.151
- 493 Cramer, M., & Keller, H. (1981). Dynamic Identification of Flows from Traffic Counts at Complex
494 Intersections. *Proceedings of the 8th International Symposium on Transportation and Traffic*
495 *Theory*, (pp. 199-209). Toronto.
- 496 Cremer, M. (1983). Determining the Time-Dependent Trip Distribution in a Complex Intersection
497 for Traffic Responsive Control. *IFAC Proceedings Volumes*, 16.4, pp. 141-147.
- 498 Cremer, M., & Keller, H. (1984). *A Systems Dynamics Approach to the Estimation of Entry and*
499 *Exit O-D Flows*. The 9th International Symposium of Transportation and Traffic Theory. Utrecht,
500 the Netherlands: VUN Science Press.

- 501 Cremer, M., & Keller, H. (1987). A new class of dynamic methods for the identification of origin-
502 destination flows. *Transportation Research Part B: Methodological*, 21(2), 117-132.
- 503 Denney Jr., R. W., Curtis, E., & Olson, P. (2012). The national traffic signal report card. *ITE*
504 *Journal*, 82(6), 22-26.
- 505 Federal Highway Administration (FHWA). (2019). Automated Traffic Signal Performance
506 Measures (ATSPMs). Retrieved January 17, 2019, from Center for Accelerating Innovation:
507 https://www.fhwa.dot.gov/innovation/everydaycounts/edc_4/atspm.cfm
- 508 Ghanim, M. S., & Shaaban, K. (2019, May). Estimating Turning Movements at Signalized
509 Intersections Using Artificial Neural Networks. *IEEE Transactions on Intelligent Transportation*
510 *Systems*, 20(5), 1828-1836. doi:10.1109/TITS.2018.2842147
- 511 Grant, M., Bowen, B., Matthew, D., Winick, R., Bauer, J., Chavis, A., & Trainor, S. (2011).
512 Congestion Management Process: A Guidebook. ICF International, Inc., Motion Maps, LLC,
513 Science Applications International Corporation. Washington, D.C.: U.S. DOT.
- 514 Hauer, E., Pagitsas, E., & Shin, P. (1981). Estimation of Turning Flows from Automatic Counts.
515 *Transportation Research Record*, 795, 1-7.
- 516 Hillier, F. S., & Liebeman, G. J. (2010). *Introduction to Operation Research*. New York: McGraw-
517 Hill Higher Education.
- 518 Hollander, Y., & Liu, R. (2008). The principles of calibrating traffic microsimulation models.
519 *Transportation*, 35(3), 347-362. doi:10.1007/s11116-007-9156-2
- 520 Hourdakis, J., Michalopoulos, P. G., & Kottommannil, J. (2003). A practical procedure for
521 calibrating microscopic traffic simulation models. *Transportation Research Record*, 1852(1), 130-
522 139.
- 523 Huang, T., Poddar, S., Aguilar, C., Sharma, A., Smaglik, E., Kothuri, S., & Koonce, P. (2018,
524 December). Building Intelligence in Automated Traffic Signal Performance Measures with
525 Advanced Data Analytics. *Transportation Research Record*, 2672(18), 154-166.
526 doi:10.1177/0361198118791380
- 527 Johnson, D. S. (1993). *Network Flows and Matching: First DIMACS Implementation Challenge*.
528 (D. S. Johnson, & C. McGeoch, Eds.) Boston, MA, USA: American Mathematicsl Soc.
- 529 Kennington, J. L., & Helgason, R. V. (1980). *Algorithms for Network Programming*. New York:
530 A Wiley-Interscience Publication.
- 531 Knoop, V. L., Hoogendoorn, S. P., & van Zuylen, H. J. (2009, January 1). Processing Traffic Data
532 Collected by Remote Sensing. *Transportation Research Record*, 2129(1), 55-61.
533 doi:10.3141/2129-07
- 534 Lu, Z., Rao, W., Wu, Y.-J., Guo, L., & Xia, J. (2015). A Kalman filter approach to dynamic OD
535 flow estimation for urban road networks using multi-sensory data. *Journal of Advanced*
536 *Transportation*, 49, 210-227.

537 Martin, P. T. (1995). Turning Movement Estimation in Real Time (TMERT). California
538 Polytechnic State University, San Luis Obispo, Department of Civil Engineering. California
539 PATH Research Report UCB-ITS-PRR-95-29.

540 Martin, P. T. (1997, July/August). Turning Movement Estimation in Real Time. Journal of
541 Transportation Engineering, 123(4), 252-260.

542 Martin, P. T., & Bell, M. C. (1992). Network Programming to Derive Turning Movements from
543 Link Flows. Transportation Research Record 1365, 147-154.

544 Nihan, N. L., & Davis, G. A. (1987). Recursive Estimation of Origin-Destination Matrices from
545 Input/Output Counts. Transportation Research B: Methodological, 21(2), 149-163.

546 Reed, T. (2019). Global Traffic Scorecard. Kirkland, WA, U.S.A.: INRIX Research.

547 Schaffer, M. C. (1988, October). Estimation of Intersection Turning Movements from Approach
548 Counts. ITE Journal, 58(10), 41-46.

549 Shiravi, S. (2019). Clear Signals: How to Use ATSPMs to Keep Your Intersections Healthy.
550 Ontario: Miovision Technologies Inc. Retrieved from <https://www2.miovision.com/atspm-ebook>

551 Shoup, G., Remias, S. M., Hainen, A. M., Grimmer, G., & Davis, A. D. (2013, March).
552 Characterizing Reliability of Manual Intersection Turning Movement Counts Using Modern Data
553 Collection Technology. Joint Transportation Research Program Other Publications and Reports.
554 Retrieved June 30, 2019, from <https://docs.lib.purdue.edu/jtrpdocs/6>

555 Sorenson, H. (1970). Least-Squares Estimation: from Gauss to Kalman. IEEE Spectrum, 63-68.

556 Transportation Research Board. (2010). HCM 2010: Highway Capacity Manual. Washington,
557 D.C.

558 U.S. Census Bureau. (2018). QuickFacts: Orem City, Utah. Retrieved September 17, 2018, from
559 <https://www.census.gov/quickfacts/fact/table/oremcityutah/PST045217>

560 Utah Department of Transportation (UDOT). (2019). Signals, 4.2.2. (UDOT) Retrieved June 27,
561 2019, from Automated Traffic Signal Performance Measures: <https://udottraffic.utah.gov/atspm>

562 van der Zijpp, N. (1996). Dynamic Origin-Destination Matrix Estimation on Motorway Networks,
563 Ph.D. Thesis. Department of Civil Engineering, Delft University of Technology.

564 van Zuylen, H. (1979). The Estimation of Turning Flows on a Junction. Traffic Engineering
565 Control, 20(11), 539-541.

566 Yang, Z., & Pun-Cheng, L. S. (2018). Vehicle detection in intelligent transportation systems and
567 its applications under varying environments: A review. Image and Vision Computing, 69, 143-
568 154. doi:10.1016/j.imavis.2017.09.008

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574 **Table 1.** Calculated R² for different weight sets (Orem, Utah Network)

Period	R ² (%)				
	W0	W1	W2	W3	W*
1	75.0	85.2	89.9	87.5	92.2
2	77.3	85.2	89.9	86.9	92.8
3	75.8	89.4	90.6	91.2	94.1
4	79.5	85.5	88.6	87.7	91.9
5	80.2	87.6	87.4	88.0	93.0
6	76.3	81.3	89.9	84.1	91.7
7	79.5	87.9	89.8	89.3	92.4
8	78.9	88.0	90.3	88.5	92.9
9	75.5	86.4	89.5	86.4	90.4
10	84.3	89.9	91.6	90.4	93.5
11	82.2	88.6	92.8	90.6	93.7
12	81.1	90.1	92.3	90.7	93.1
Mean	78.8	87.1	90.2	88.4	92.7
Std. Dev.	2.8	2.4	1.4	2.0	1.0
Min	75.0	81.3	87.4	84.1	90.4
Max	84.3	90.1	92.8	91.2	94.1

Note: W0, W1, W2, W3- Martin, P.T. (1995)- SLO Weights
W*- Optimal Weights

575 **Table 2.** Weight sets used in the validation

Arc	Weight Sets				
	W0	W1	W2	W3	W*
Left Protected	1	3	6	3	60
Right	1	3	6	4	48
Through	1	2	3	2	1
Error	3	15	30	20	1
Link	1	1	1	1	1
Left Permitted	1	5	9	7	60

Note: W0, W1, W2, W3- Martin, P.T. (1995)- SLO Weights
W*- Optimal Weights

576 **Table 3.** Calculated estimation reliability parameters for optimal weights (W*) and actual CPU
 577 time (Orem, Utah Network)

Period	R ² (%)				Overall RMSE (# veh.)	Overall r	Actual CPU Time (ms)
	L	T	R	Overall			
1	0.7	95.7	39.5	92.2	9.0	0.96	9
2	35.0	96.9	50.1	92.8	8.8	0.96	7
3	37.8	97.6	69.5	94.1	8.2	0.97	6
4	29.8	96.5	37.6	91.9	9.2	0.96	4
5	21.6	96.6	42.3	93.0	9.6	0.96	6
6	8.0	95.7	39.5	91.7	8.7	0.96	7
7	36.1	97.7	13.6	92.4	8.8	0.96	6
8	37.9	97.4	49.8	92.9	8.7	0.96	7
9	7.0	94.1	23.4	90.4	9.7	0.95	5
10	55.4	96.9	43.1	93.5	9.1	0.97	7
11	41.7	98.0	56.8	93.7	8.6	0.97	6
12	31.6	97.1	35.7	93.1	9.0	0.96	6
Mean	28.5	96.7	41.7	92.7	8.9	0.96	6
Std. Dev.	15.5	1.0	13.9	1.0	0.4	0.01	1
Min	0.7	94.1	13.6	90.4	8.2	0.95	4
Max	55.4	98.0	69.5	94.1	9.7	0.97	9

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